

# **Investigation and Mitigation Report on Incident Occurring at**

**125 CROWN TERRACE**

**December 16, 2013**



**Prepared by**

**Tom C. Hui, S.E., C.B.O.**

**Department of Building Inspection**

**City & County of San Francisco**

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**SUBJECT:** Briefing on 125 Crown Terrace project  
**PREPARED BY:** Tom C. Hui, S.E., C.B.O., Director  
San Francisco Department of Building Inspection  
**DATE:** February 5, 2014

### **INCIDENT**

On December 16, 2013, at approximately 10:30 p.m., the single-family house at 125 Crown Terrace in San Francisco's Twin Peaks neighborhood fell off its site preparation shoring and slid down the property slope. Scheduled construction work for the day had stopped about 5 p.m.

The 1947 one-story wood-framed structure with a large crawl space below was under construction for a major remodel when the failure occurred. The structure had been raised off its foundation for the construction project and placed atop temporary supports. A steel cradle beam that spanned between two temporary wood cribbing towers buckled. That caused the house to come off its shoring and to slide downhill toward Graystone Terrace, forcing the temporary evacuation of a neighboring home. No injuries were reported. The house remained within its property lot; no adjacent buildings were damaged.

Emergency shoring work began on December 17, 2013. The City Attorney called on DBI to halt the emergency work, pending investigation of the site (Attachment H). The City Attorney's office gave its OK to lift the stop-work order on December 20, 2013 (Attachment H), allowing the temporary shoring to prevent any additional slippage and to make the compromised structure safe for the time being.

### **BACKGROUND**

The San Francisco Department of Building Inspection (DBI) issued a building permit for structural shoring, structural additions and soil stabilization on November 28, 2012 (Attachment N). DBI acted on the permit after the project was approved by the Planning Commission in October 2012 and upheld by the Board of Appeals over the objection of some neighbors in May 2013, and after the construction details were reviewed and approved by the independent Structural Advisory Committee (SAC), as required by ordinance in designated steep slope neighborhoods. SAC is a five-member independent body comprised of an architect, geotechnical engineer, geologist, shoring engineer and structural engineer.

At the time of the structural failure, a lower retaining wall adjacent to Graystone Terrace and a segment of the upper wall below Crown Terrace already had been installed.





## **FINDINGS**

The structural collapse and slide occurred because the developer, Pattani Construction, Inc., managed by Mel Murphy:

- 1) failed to follow and implement the approved plans and the sequence of construction as stipulated and outlined in the DBI permit, and
- 2) failed to follow and implement the special inspection instructions, as mandated under the approved DBI permit and the State of California Building Code. As a result, the required independent inspection of the shoring work was not conducted, leaving an unsafe condition undiscovered until after the collapse occurred.

The combination of these two factors contributed to a site situation where one of the steel support beams gave way. Photos of the construction site hours before the failure suggest the work performed was dramatically different than the approved plan. Had the contractor brought in inspectors during installation of the shoring structure, the work would have been halted prior to the collapse.

The findings were based on a report submitted by the project developer's engineer of record, Santos & Urrutia Structural Engineers, Inc., (Attachment D), field inspections and document reviews by DBI engineers (Attachment G), and a third-party peer review by Steven Lew, a structural engineer with the firm Tuan and Robinson Structural Engineers, Inc., and his colleague, shoring engineer, Timothy Mathison, P.E. (Attachment F), who served on the Structural Advisory Committee originally convened for the 125 Crown Terrace project.

Rodrigo Santos, the engineer of record for 125 Crown Terrace, testified at the January 15, 2014 Building Inspection Commission meeting that the developer's entire design and construction team took full responsibility for the structural failure. "DBI did everything right and took all proper steps. This was a failure to implement the approved plans. This accident could have been prevented, and we are very fortunate no one was injured," Mr. Santos testified.

## **NEXT STEPS**

- 1) **FEES AND PENALTIES:** Prior to any future work commencing, the developer must pay an additional \$24,181 in permit fees for work valued at \$72,198 that was performed without permit; and a \$6,010.71 penalty for failing to set up a shoring structure as stipulated in the approved plans. The fine -- the maximum allowable -- is based upon the correct valuation of the work done without permit multiplied by a factor of nine (9). The penalty schedule is outlined in the San Francisco Building Code and stipulated in Sec. 110A, table 1A-K.  
Total: \$30,191.71



- 2) **SAFETY MANDATE:** As recommended by the DBI Director, the developer must immediately provide a design and obtain a permit to install a temporary surface drainage system to control surface runoff, and to protect the job site's stability from winter storm conditions. The developer's geotechnical engineer will design the system. Once submitted, DBI will review the plan, as will Frank Rollo, the geotechnical representative from the independent SAC. Per the recommendations outlined by Rollo (See Attachment P), the owner's special inspectors and engineer will provide DBI with daily reports, including photos, and follow all other recommended steps to document correct implementation of this surface drainage plan. DBI also will conduct unscheduled site inspections to make sure the contractor adheres to the approved plan. DBI will issue a stop-work order if the plan is not followed.
- 3) Once steps One and Two are satisfied, the developer must start installing a new foundation, which is an essential part of the required shoring and detailed in DBI's approved permit application.
- 4) Throughout construction, DBI will conduct unannounced inspections, and the developer will be required to pay for those inspections. In addition, the developer's engineer of record and geotechnical engineer must observe and supervise the installation on a full-time basis.
- 5) Going forward, any DBI inspections of the property will be overseen by a deputy director and performed by a team that includes the district inspector assigned to the Twin Peaks area, and either the chief building inspector, or the senior building inspector.
- 6) Future permitting requirements for construction of the house to proceed:
  - To continue with construction of the house, which was damaged during the collapse, the developer must file a new permit application with DBI.
  - DBI then will forward the application to City Planning to determine whether the revised plan is a demolition or remodel. If the determination is made that the project constitutes a remodel, then Planning staff would decide whether to approve the plan at staff level or send the plan to the Planning Commission for a hearing.
  - If the determination is made that the project constitutes a demolition, then the developer must revise the application and file a Form 6 total demolition permit, and a Form 2 new building permit. Planning then would decide whether or not its commission should hear the matter.
  - DBI will review and approve the permit application once and if the Planning Department/Commission grants approval. The DBI director will retain the right to reconvene the Structural Advisory Committee to review the plan and make recommendations. If DBI approves the permit, opponents would have 15 days to file an appeal with the City's Board of Appeals.



- Once the appeals process is complete, and if a permit is issued, construction of the house then could resume.
- The developer will be subject to special inspections protocol for any future shoring and foundation work, with the engineer of record and geotechnical engineer required to submit to DBI weekly reports and photos. In addition, DBI will carry out unannounced spot-check inspections, to be paid for by the developer.

### **PERMIT PROCESS UNDER REVIEW**

Discrepancies were found between the developer's valuation of the building site plan permit issued on November 28, 2012 and calculations made in the Marshall & Swift construction-cost calculation tables, which are widely used by building departments throughout the United States to quantify appropriate permit fees.

The developer originally estimated the cost in October 2011 at \$60,000 on his submitted permit application form. A DBI plan checker revised the cost to \$300,000 on November 26, 2012 as part of the standard site plan permit review. The value was again revised by DBI to \$610,500 on December 9, 2013 due to a site plan review prompted by a filed complaint. On December 31, 2013 DBI's Principal Engineer reviewed the approved plans/drawings in detail and revised the cost of construction to \$1,570,000 (Attachment G).

Investigations into the permit valuation discrepancies were initiated following the structural failure on the site (See INVESTIGATIONS section below). In addition, DBI Director Tom Hui ordered additional mandatory construction-cost valuation training for all staff involved in the related permitting and plan-checking processes. Random audits will be conducted to ensure the ongoing integrity of the permitting process. In addition, the Department will implement the six other recommendations set forth in this report (See NEXT STEPS section).

### **INVESTIGATIONS**

- 1) DBI engineers, in consultation with a peer review, investigated the December 16, 2013 collapse of the structure at 125 Crown Terrace. (Completed, see findings above.)
- 2) The City Attorney's Office is conducting an investigation, and has requested documents related to the structural collapse and the issuance of the building permit, plan-checking, engineering, project management and inspections related to 125 Crown Terrace. (Pending.)
- 3) The General Services Administration, Human Resources division, conducted an independent investigation into the permitting process, plan-checking and inspections as they are related to 125 Crown Terrace and whether any DBI and/or City and County of San Francisco protocols or policies were violated by City personnel. (Completed.)











City and County of San Francisco  
Department of Building Inspection



Edwin M. Lee, Mayor  
Tom C. Hui, S.E., C.B.O., Director

## CHRONOLOGY OF EVENTS

**SUBJECT:** 125 Crown Terrace, San Francisco

**PREPARED BY:** Tom C. Hui, S.E., C.B.O., Director

1. CCSF Alert dated December 17, 2013 of a house under construction that collapsed at 10:41p.m.
2. December 17, 2013
 

6:22 a.m.	Joe Duffy texted me regarding this incident.
9:00 a.m.	City Engineer (Fuad Sweiss) and Ray Lui, DPW called me and William Strawn. City Engineer suggested DBI declare an emergency demolition of subject building.
11:00 a.m.	William Strawn and I attended the ground-breaking event for 299 Fremont. We met the Channel 5 reporter at the door and briefed Mayor Lee on DBI's action plan.
1:00 p.m.	Engineer of Record obtained the emergency shoring permit issued by the 5 <sup>th</sup> Floor plan review counter.
2:00 p.m.	Dan Lowrey, William Strawn, and I visited the site and were interviewed by reporters. The Engineer of Record, Albert Urrutia, came to the site at 3:00 p.m. for structural observation. The contractor started to work on the shoring.
3. December 18, 2013
 

9:00 a.m.	BIC Meeting: This project was discussed with the Commission. President McCarthy expressed concerns on the urgency for corrective actions.
12:30 p.m.	City Attorney Dennis Herrera called DBI to stop all construction on the site, pending City Attorney

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investigation per the Building Inspection Commission's request. His office followed up with a formal letter to DBI

4. December 19, 2013

9:00 a.m.

I sent out DBI's Action Plan to the Building Inspection Commission.

12:00 p.m.

Dan Lowrey and I went to the ground breaking event for 333 Brannan, and I informed Mayor Lee of DBI's Action Plan.

5. December 20, 2013

8:15 a.m.

I discussed with DBI Principal Engineer Hanson Tom the implementation of DBI's Action Plan.

3:41 p.m.

Yvonne Mere from the City Attorney's Office sent a letter to DBI withdrawing the Stop Work Order and enabling partial demolition to temporarily secure and stabilize the building structure at the site.

6. December 24, 2013

8:15 a.m.

I met with staff including DBI inspectors Dan Lowrey, Patrick O'Riordan and Matthew Greene, and DBI engineers Hanson Tom, Rudy Pada, and Robert Chun to remind them to perform their professional duties on all projects with fairness.

7. December 31, 2013

8:30 a.m.

I met with Hanson Tom, Ed Sweeney, William Strawn, and Angus McCarthy, to discuss the reports from Santos & Urrutia on this incident.

8. January 2, 2014

9:30 a.m.

Meeting between Hanson Tom, Ed Sweeney, Patrick Buscovich, and Rodrigo Santos to discuss DBI's concerns.



**MEMORANDUM**

**DATE:** December 19, 2013

**TO:** Honorable Members of the Building Inspection Commission

**FROM:** *Tom C. Hui*  
Tom C. Hui, S.E., C.B.O.  
Director

**SUBJECT:** **DBI Action Plan to Investigate the 125 Crown Terrace Building  
Collapse of December 16, 2013**

As I proposed to the Building Inspection Commission at yesterday's Commission meeting, I am taking the following steps to understand what caused the construction site incident at 125 Crown Terrace, and what the mitigation plan will be to restore the structure to the newly-installed foundation.

- First:** The property owner's engineering firm Santos & Urrutia, will provide to DBI by 3:00 p.m. on Monday, December 23, 2013, its technical report explaining the cause of the structure's collapse and slide down the property slope. This report also will include the engineering firm's proposed mitigation plan, and timeline, to place the structure back upon its newly-poured foundation.
- Second:** The Director instructed the engineer of record to prepare an investigation report to explain the cause of the incident and what is the proposed mitigation and restoration plan in the future by no later than Monday, December 23, 2013. In addition to DBI engineers' review and professional critique of the owner's proposed mitigation and restoration plan, the Director ordered DBI's Deputy Director Edward Sweeney and DBI's Principal Engineer, Hanson Tom, to have a third-party structural engineering peer review to evaluate the technical feasibility of the report. The third-party's structural engineering peer review shall be done by Tim Mathison and Steve Lew of the original peer review group.

With the holidays upon us, we expect this third-party structural peer group evaluation to occur and to provide the DBI Director with its evaluation by no later than December 31, 2013. Once the Director has both this third-party professional report, and the evaluation from DBI's own engineers, the Director will then decide whether or not the owner's mitigation and restoration plan is technically feasible.



- Third:** If the Director's decision is to implement the approved mitigation and restoration plan, Director Hui will have the Deputy Director for Inspections, a Chief Building Inspector and the District Director to be involved and monitoring, daily, the implementation of any approved work plan for this project. These inspections will ensure that all building code requirements are met, and that what is proposed in the approved plans can be implemented in the actual field conditions presented by the job site.
- Fourth:** The Director has ordered the Deputy Director for Permit Services, which includes oversight of DBI's professional plan review services, to work closely with Hanson Tom and DBI's Manager of Technical Services, who also is certified as a Marshall & Swift construction cost evaluator, and to make calculations of this specific project's valuation compared to other similar projects in San Francisco. Marshall & Swift is recognized, and used actively, by building departments throughout the United States to obtain accurate project and construction cost valuations. DBI has long utilized the Marshall & Swift published tables to examine project valuation estimates made by the property owner's contractors, and to provide trusted metrics to DBI Plan Review staff when evaluating whether or not submitted project valuations are accurate. When any discrepancy is found between the owner's estimate and the Marshall & Swift calculations, DBI issues a correction notice to the owner and revises the estimate to reflect accurate construction cost numbers for the location and local market.
- Fifth:** If all the technical engineering issues are resolved, the new application will be reviewed by DBI and Planning.

In addition to the above DBI Action Plan, please note that per a request by BIC Commissioner Walker, the City Attorney began a separate investigation into the 125 Crown Terrace construction incident. DBI is cooperating fully with the City Attorney's inquiry and welcomes this additional evaluation by the City's legal experts.

cc: Mayor Edwin M. Lee  
Supervisor Scott Wiener  
Steve Kawa, Mayor's Office  
John Rahaim, Director of City Planning  
Edward Sweeney, DBI Deputy Director of Permit Services  
Dan Lowrey, DBI Deputy Director of Inspection Services  
Hanson Tom, DBI Principal Engineer

City and County of San Francisco  
Department of Building Inspection



Edwin M. Lee, Mayor  
Tom C. Hui, S.E., C.B.O., Director

## MEMORANDUM

**DATE:** December 31, 2013

**TO:** Honorable Members of the Building Inspection Commission

**FROM:** *Tom C. Hui*  
Tom C. Hui, S.E., C.B.O.  
Director

**SUBJECT:** Update of DBI Action Plan for 125 Crown Terrace and 3418 - 26<sup>th</sup> Street

I would like to acknowledge and extend my appreciation to my staff who have been working hard on reviewing these projects during the past two weeks. It is my top priority to take all appropriate steps to ensure our permit review and inspection processes are transparent and to treat all customers fairly and equitably.

I have instructed Daniel Lowrey, Deputy Director, to oversee future building inspection for both projects at 125 Crown Terrace and 3418 - 26<sup>th</sup> Street. It will be required to have the district inspector, along with the chief building inspector or senior building inspector, to perform future inspections. All communications, including inspection scheduling, shall be handled by the Inspection Services clerks on the 3<sup>rd</sup> Floor.

I also have asked Hanson Tom, DBI Principal Engineer and Edward Sweeney, DBI Deputy Director, to review and comment upon the mitigation and restoration report for 125 Crown Terrace that was submitted by the engineer of record on Monday, December 23.

The third-party independent structural engineering peer review will be performed by Tim Mathison and Steve Lew of the original peer review group. They are among the best structural engineers in the Bay Area. The Department is still waiting for the independent SAC report, prior to issuing its final report. The final report should be ready by next week.

In addition to the report review and evaluation, I have asked Hanson Tom to evaluate the construction cost of Crown Terrace for both original and emergency shoring permits.

The new application shall be submitted to Planning and the Department of Building Inspection for review once all the technical engineering issues have been resolved.

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cc: Mayor Edwin M. Lee  
Supervisor Scott Wiener  
Steve Kawa, Mayor's Office  
John Rahaim, Director of City Planning  
Edward Sweeney, DBI Deputy Director of Permit Services  
Dan Lowrey, DBI Deputy Director of Inspection Services  
Hanson Tom, DBI Principal Engineer

December 23, 2013

Mr. Hanson Tom  
Department of Building Inspection  
City of San Francisco  
1660 Mission Street  
San Francisco, CA 94103

Re: Structure Collapse: 125 Crown Terrace, San Francisco, CA

Complaint No.: 201343501 & 201343861  
S&U Job: 9356  
Block 2719B, Lot 003  
Subject: Engineering Report

Mr. Hanson Tom,

At Director Tom Hui's request this is the engineering investigative report on 125 Crown Terrace.

### **Background**

The original structure is one story of timber framing over a large crawl space. Under permit application 2011/10/06/6315, the building is in a process of being remodeled and added to. The addition is both vertical and horizontal, with the design adding floors under the existing structure. The permit was issued at the end of November 2013, which included both soil and structure shoring.

Construction commenced in the beginning of December 2013. In this process the building was placed on beams and cribbing with the lower crawl space walls removed. The house moving contractor decided that it was better to leave the building heavy (with plaster and finishes) in order to lessen the potential for wind uplift. The lower retaining walls were placed, the upper shoring beams and a portion of the upper wall was placed. In order to place the required retaining walls the beams and cribbing needed to be moved and readjusted several times.

On Monday, December 16, 2013 the contractor was in the process of moving the shoring towers to their latest locations. The building was supported by a beam to the north with three towers, a beam to the south on two towers and was in the process of digging the pit for the middle (future) tower. At 5pm of that day construction ceased, the excavator was left in place, under the house, with its arm in an inverted V.

## **The Failure**

At 10:30pm, Monday, December 16, 2013, the south beam buckled, pulling the house off its cribbing toward the south. This pulled the north cribbing beam off its towers and the structure started to slide with the steel. At this point the eastern most portion of the structure landed and impaled itself on the arm of the excavator. Because the excavator was on a flat stable spot it did not move. This arrested the down hill movement of the structure. At this point the structure broke into three distinct sections; one eastern piece on the excavator and two western pieces resting on the eastern one. The southern most piece was listing towards the property to the directly to the south. In the attached calculations, they show that the southern beam was overstressed to 159% of allowable.

## **Temporary Stabilization**

On December 17<sup>th</sup>, 2013, Santos & Urrutia obtained permit application 2013/12/17/4398 in order to provide emergency demolition of the southern most section of the building which was placing the greatest pressure on the excavator and was listing. The exterior walls were removed and retained for future use. Then the remaining portions were stabilized by chaining them back to the existing vertical soil shoring piles. The northern most and eastern portions were lightened (plaster and finishes removed) and a shear wall was added to the northern most section.

This stabilization is not recommended for any long term as weather can further destabilize the site.

## **Mitigation**

In order for construction to proceed, we will remove the walls of the eastern most section and retain them for reuse in their original position in the final structure, exposing the excavator. Stabilize the northern most section and move it to a flat space on the site. Demolish the remaining portions of the floor and remove the excavator. In the final design the walls that were retained (during the stabilization and mitigation) will be placed back to their proposed original locations.

Should you have any questions, please contact me.

Sincerely,

  
Albert Urrutia  
Structural Engineer



Attachments: Collapse Temporary Shoring Plan, Beam Stress Calculations, Photos, Emergency Demolition and Stabilization Plan and Mitigation Plan

# COLLAPSED TEMP SHORING PLAN

TEMPORARY SHORING PLAN  
125 CROWN TERRACE  
SAN FRANCISCO, CALIFORNIA

Date: 12/20/13  
Scale: 1/16"=1'-0"  
Drawn By: A.U.  
Job No: 9356  
Sheet: S1  
Of 1 Sheets

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GRAYSTONE TERRACE  
SIDEWALK

PROPERTY LINE

PROPERTY LINE

PROPERTY LINE

125 CROWN TERRACE  
SUBJECT PROPERTY

FUTURE TOWER

FUTURE TOWER

120 GRAYSTONE TERRACE  
ADJACENT PROPERTY

NORTH

## COLLAPSED TEMP .SHORING PLAN

SCALE: 1/16"=1'-0"

115 CROWN TERRACE  
ADJACENT PROPERTY

1'-6" CONC. GRADE BEAM &  
14" DRILLED PIER SYSTEM,  
TYP.

10" RETAINING  
WALL BELOW

44' CRIBBING

FIELD EXCAVATOR  
UNDERNEATH  
BUILDING

FUTURE TOWER

W10x49

W6x15

W6x15

W6x15

W6x15

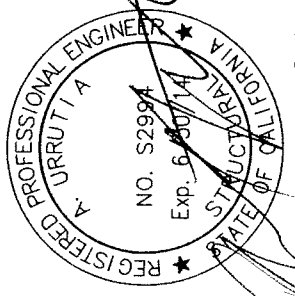
FUTURE FACE OF  
CONC. WALL

CURRENT EDGE  
OF MAT SLAB

RETAINING WALL

PROPERTY LINE

CROWN TERRACE

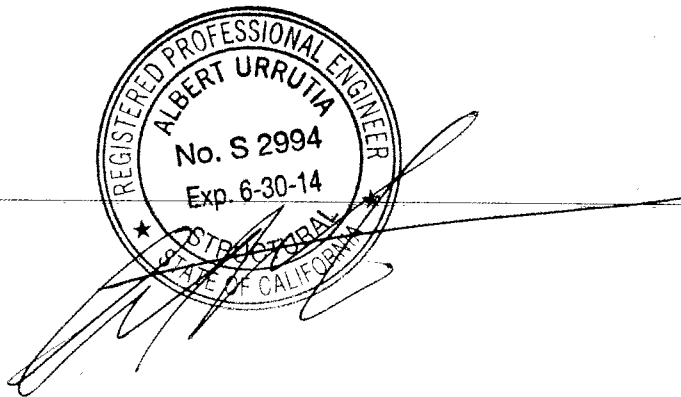


STRUCTURAL CALCULATIONS FOR:

**SHORING BEAM FAILURE  
125 CROWN TERRACE  
SAN FRANCISCO, CALIFORNIA**

REPORT PREPARED BY:

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**S & U JOB # 9356**

**DATE: December 23, 2013**

**PAGES: 2**

125 CROWN TERRACE (#9356)  
SAN FRANCISCO, CA

SHORING BEAM FAILURE

PURPOSE

THIS CALCULATION IS TO SHOW THE  
STRESS LEVEL OF THE EXISTING SOUTHERNMOST  
SHORING BEAM PRIOR TO BUCKLING  
AND COLLAPSE,

LOADS

ROOF / CEILING

ASPHALT SHINGLE ROOF	3 PSF
1 X SHEATHING	2.5 PSF
2x4 @ 16" O.C. (RAFTERS)	1.1 PSF
2x12 @ 16" O.C. (CEILING)	3.5 PSF
PLASTER + LATH	8 PSF
INSULATION	1 PSF
TOTAL	19.1 PSF

FLOOR

1 X SHEATHING	2.5 PSF
1 X HARDWOOD	3.0 PSF
2x12 @ 16" O.C.	3.5 PSF
TOTAL	9.0 PSF

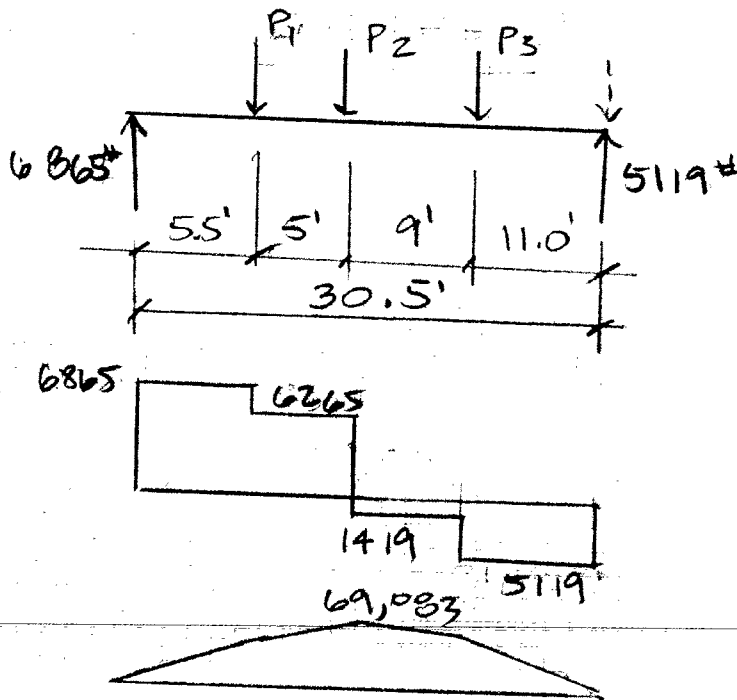


# WALLS

EXTERIOR SIDING	2.5 PSF
2x4 STUFS @ 16" o.c.	1.1 PSF
INSULATION	1.0 PSF
PLASTER	8.0 PSF
TOTAL	12.6 PSF

INTERIOR WALLS ADD 3 PSF

## LOAD TO BEAM



$$P_1 = \left(\frac{4'}{2}\right)(8')(9.0 + 19.1) + \left(8 + \frac{4 \times 4}{2}\right)(12.6) = 600\#$$

$$P_2 = P_1 + \left(\frac{10'}{2}\right)\left(\frac{26}{2} + 8\right)(3 + 9) + \left(\frac{26}{2} + 8\right)\left(\frac{22.5}{2}\right)(19.1) + \left(\frac{26}{2}\right)(12.6)8' = 7684\#$$

$$P_3 = \left(\frac{12.5 + 10}{2}\right)(3 + 9)\left(\frac{26}{2} + 6\right) + \left(\frac{12.5 + 10}{2}\right)(8')(12.6) = 3700\#$$

$$f_b = \frac{69,083(12)}{5 \times 54.6} = 15.18 \text{ ksi}$$

ALLOWABLE

$$\frac{f}{F} = \frac{30.5(12)}{2.74} = 133.5 > \sqrt{\frac{510 \times 10^3 (1.0)}{36,000}} = 119$$

$$F_b = \frac{170 \times 10^3 (1.0)}{(133.5)^2} = 9.54 \text{ ksi} \quad \text{ALLOWABLE}$$

$$\% \text{ OVERSTRESSED} = 159\%$$



Figure 1: Collapse of Structure from Crown Terrace





Figure 2: Collapse of Structure from Graystone Terrace

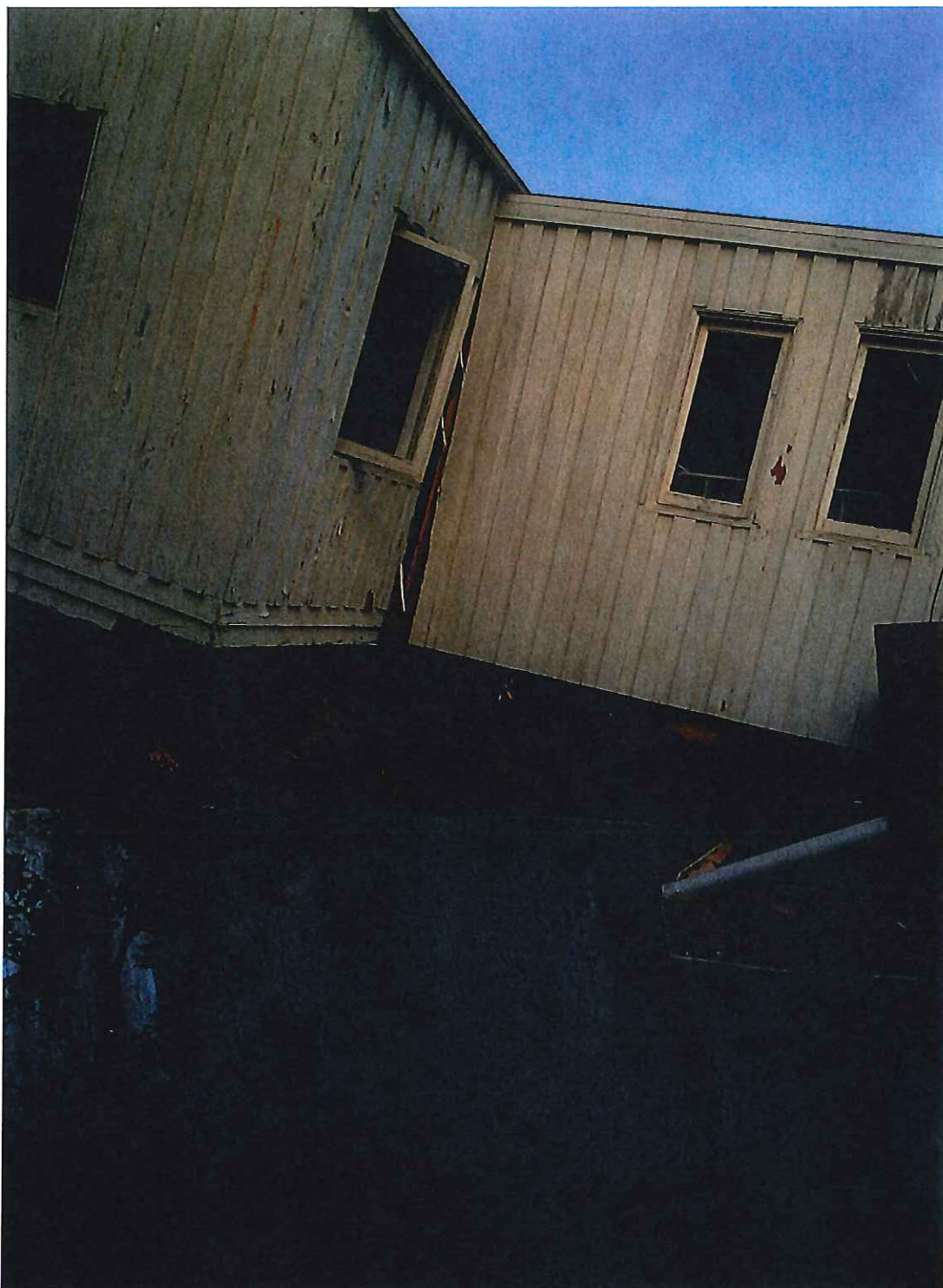


Figure 3: Collapse of Structure on Southside





Figure 4: Collapse of Structure on Southside



Figure 5: Collapse of Structure from Graystone Terrace





Figure 7: Beam Buckle Failure



Figure 8: Removal and Retention of Exterior Walls





Figure 9: Removal and Retention of Exterior Walls

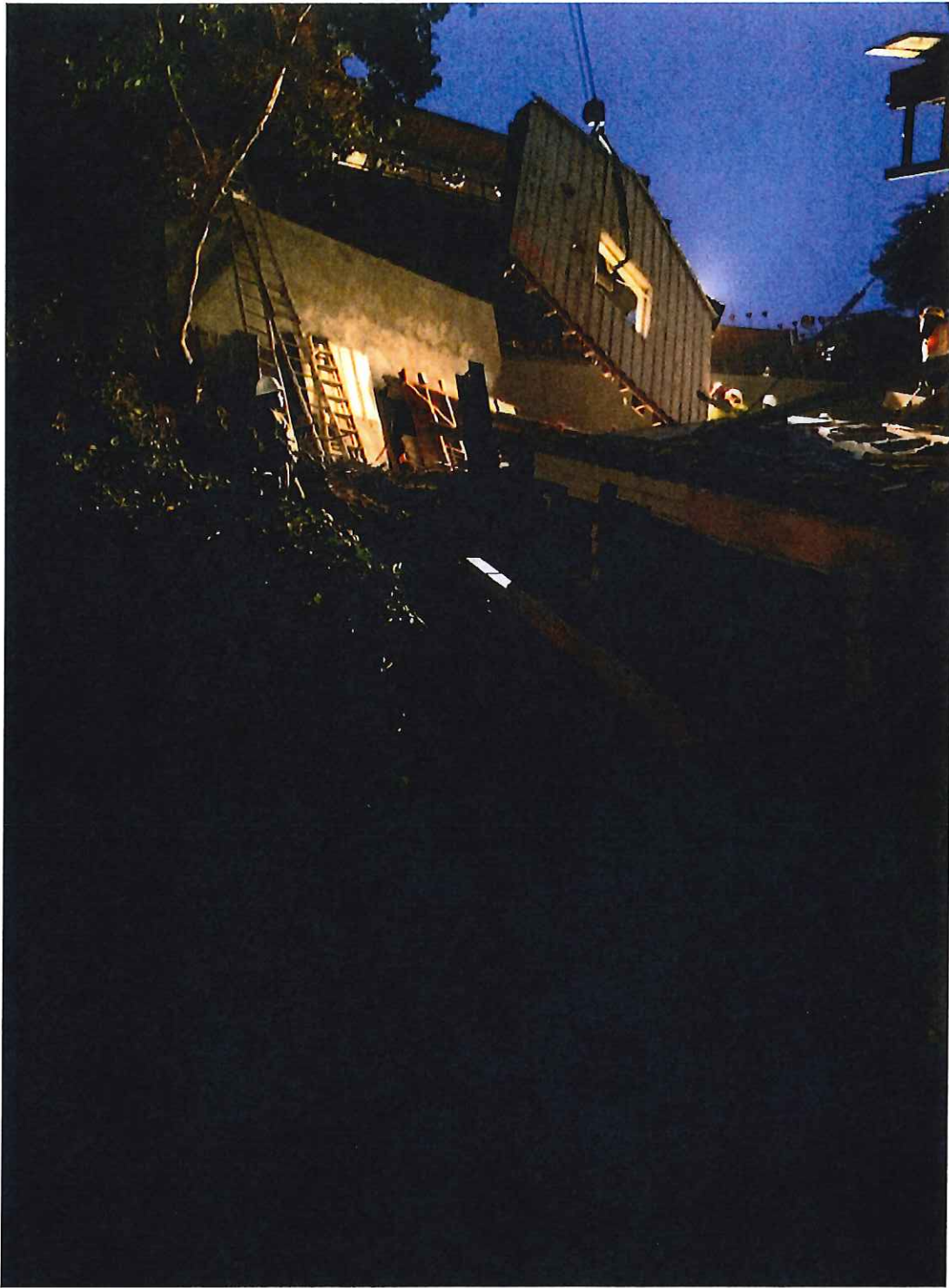


Figure 10: Removal and Retention of Exterior Walls





Figure 11: Removal and Retention of Exterior Walls



Figure 12: Removal and Retention of Exterior Walls



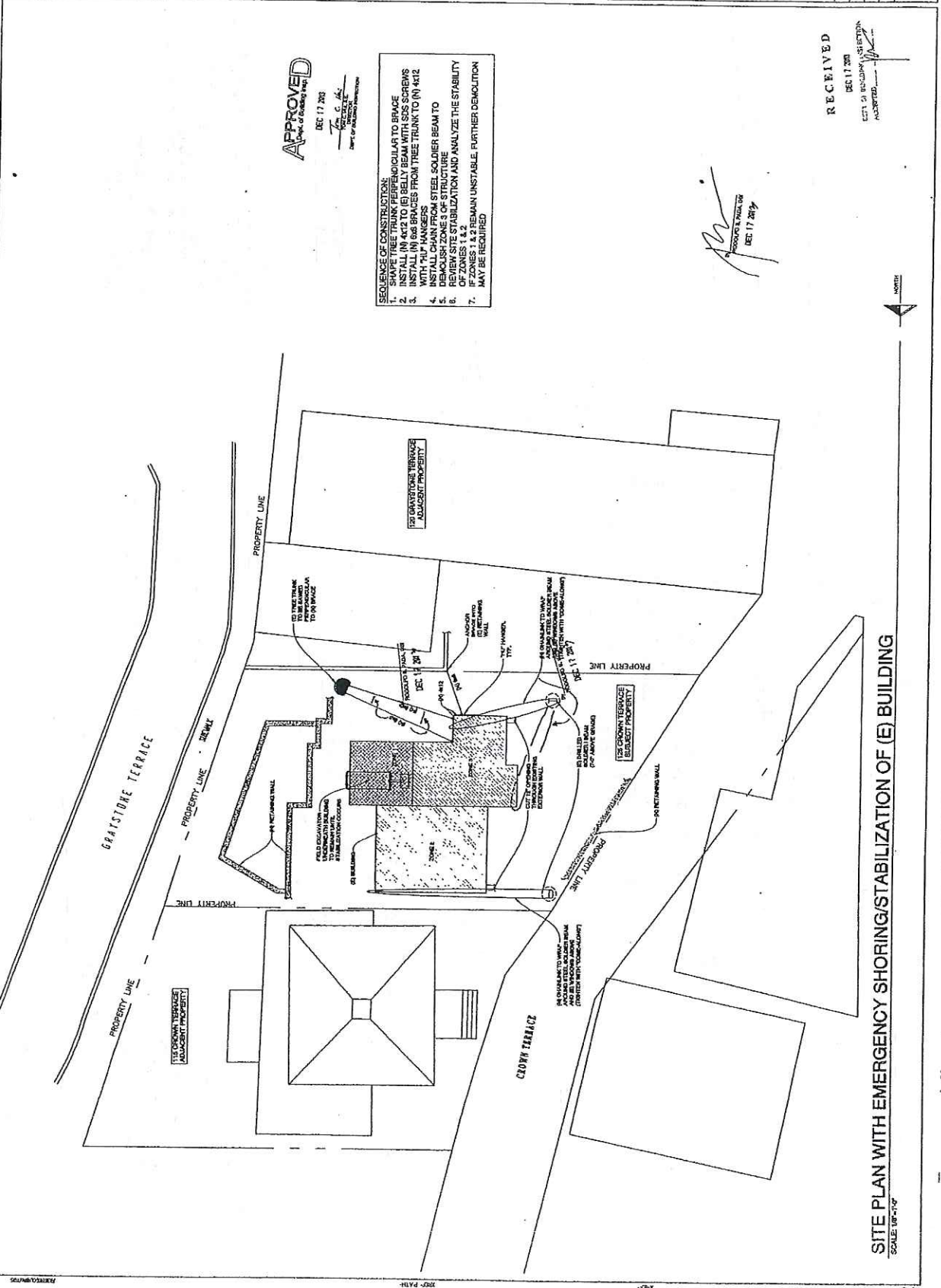


Figure 13: Removal and Retention of Exterior Walls





<b>EMERGENCY SHORING/STABILIZATION</b> <b>125 CROWN TERRACE</b> <b>SAN FRANCISCO, CA</b>		Date: 12/17/2013 Scale: 1/8" = 1'-0" Drawn by: R.S. App No: 9356 Sheet: S2 of 2 Sheets
<b>EXISTING SITE PLAN</b> <b>SEQUENCE OF EMERGENCY SHORING</b> <b>&amp; STABILIZATION OF (E) BUILDING</b>		SANTOS & URRUTIA STRUCTURAL ENGINEERS 2481 WASHINGTON STREET SAN FRANCISCO, CA 94110 FAX (415) 642-7350



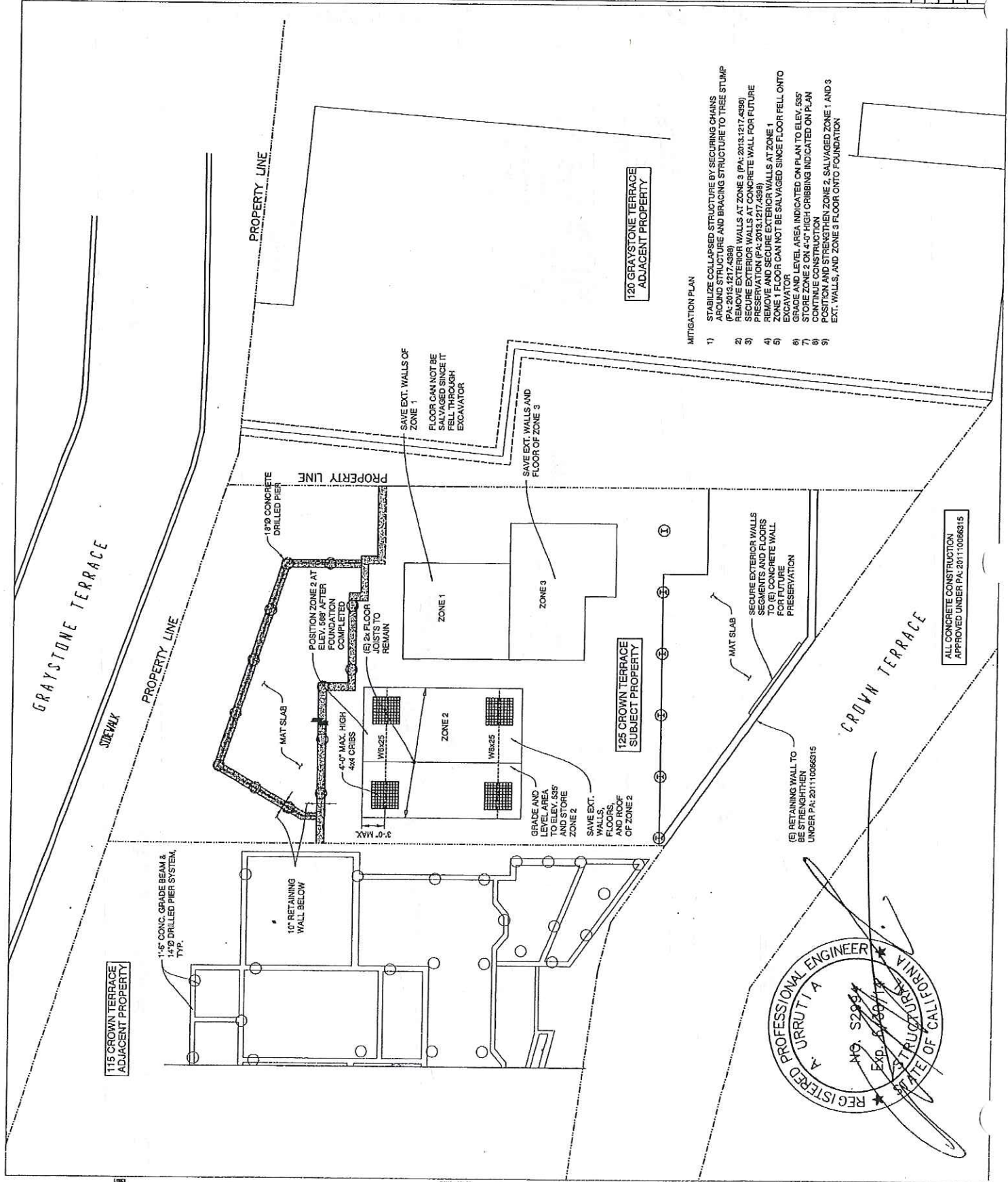


SANTI & URRUTIA  
 STRUCTURAL  
 ENGINEERS  
 101 HARRISON STREET  
 SAN FRANCISCO, CA 94102  
 TELEPHONE (415) 843-7252  
 FAX (415) 843-7260

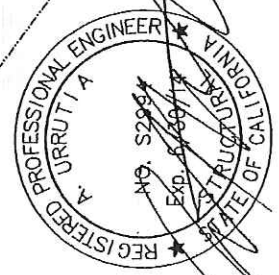
# MITIGATION PLAN

125 CROWN TERRACE  
 SAN FRANCISCO, CALIFORNIA

DATE: 12/20/13  
 SCALE: 1/16"=1'-0"  
 DRAWN BY: A.U.  
 JOB NO: 9356  
 SHEET: S1



- MITIGATION PLAN
- 1) STABILIZE COLLAPSED STRUCTURE BY SECURING CHAINS AROUND STRUCTURE AND BRACING STRUCTURE TO TREE STUMP (PA: 2013.1217.4398)
  - 2) REMOVE EXTERIOR WALLS AT ZONE 3 (PA: 2013.1217.4398)
  - 3) SECURE EXTERIOR WALLS AT CONCRETE WALL FOR FUTURE PRESERVATION (PA: 2013.1217.4398)
  - 4) REMOVE AND SECURE EXTERIOR WALLS AT ZONE 1
  - 5) EXCAVATE 1 FLOOR CAN NOT BE SALVAGED SINCE FLOOR FELL ONTO GRADE
  - 6) GRADE AND LEVEL AREA INDICATED ON PLAN TO ELEV. 535'
  - 7) STORE ZONE 2 ON 4'-0" HIGH CRIBBING INDICATED ON PLAN
  - 8) CONTINUE CONSTRUCTION
  - 9) POSITION AND STRENGTHEN ZONE 2, SALVAGED ZONE 1 AND 3 EXT. WALLS, AND ZONE 3 FLOOR ONTO FOUNDATION



ALL CONCRETE CONSTRUCTION  
 APPROVED UNDER PA: 201110066315



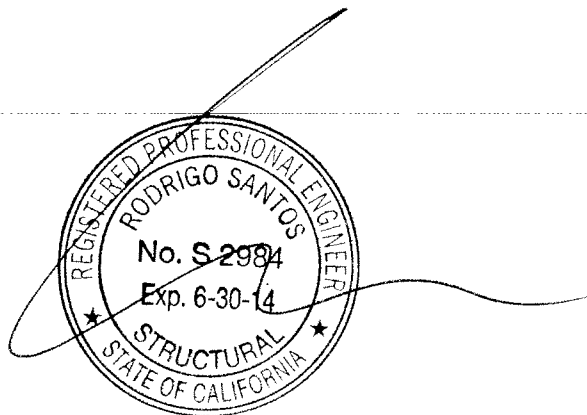
**Restoration Protocol**

*For*

**125 Crown Terrace  
San Francisco, CA 94114**

*December 30, 2013*

*Prepared by Santos & Urrutia Structural Engineers, Inc.*



## TABLE OF CONTENTS

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# SANTOS & URRUTIA



December 23, 2013

Mr. Hanson Tom  
Department of Building Inspection  
City of San Francisco  
1660 Mission Street  
San Francisco, CA 94103

Re: Structure Collapse: 125 Crown Terrace, San Francisco, CA

Complaint No.: 201343501 & 201343861  
S&U Job: 9356  
Block 2719B, Lot 003  
Subject: Engineering Report

Mr. Hanson Tom,

At Director Tom Hui's request this is the engineering investigative report on 125 Crown Terrace.

## Background

The original structure is one story of timber framing over a large crawl space. Under permit application 2011/10/06/6315, the building is in a process of being remodeled and added to. The addition is both vertical and horizontal, with the design adding floors under the existing structure. The permit was issued at the end of November 2013, which included both soil and structure shoring.

Construction commenced in the beginning of December 2013. In this process the building was placed on beams and cribbing with the lower crawl space walls removed. The house moving contractor decided that it was better to leave the building heavy (with plaster and finishes) in order to lessen the potential for wind uplift. The lower retaining walls were placed, the upper shoring beams and a portion of the upper wall was placed. In order to place the required retaining walls the beams and cribbing needed to be moved and readjusted several times.

On Monday, December 16, 2013 the contractor was in the process of moving the shoring towers to their latest locations. The building was supported by a beam to the north with three towers, a beam to the south on two towers and was in the process of digging the pit for the middle (future) tower. At 5pm of that day construction ceased, the excavator was left in place, under the house, with its arm in an inverted V.

### **The Failure**

At 10:30pm, Monday, December 16, 2013, the south beam buckled, pulling the house off its cribbing toward the south. This pulled the north cribbing beam off its towers and the structure started to slide with the steel. At this point the eastern most portion of the structure landed and impaled itself on the arm of the excavator. Because the excavator was on a flat stable spot it did not move. This arrested the down hill movement of the structure. At this point the structure broke into three distinct sections; one eastern piece on the excavator and two western pieces resting on the eastern one. The southern most piece was listing towards the property to the directly to the south. In the attached calculations, they show that the southern beam was overstressed to 159% of allowable.

### **Temporary Stabilization**

On December 17<sup>th</sup>, 2013, Santos & Urrutia obtained permit application 2013/12/17/4398 in order to provide emergency demolition of the southern most section of the building which was placing the greatest pressure on the excavator and was listing. The exterior walls were removed and retained for future use. Then the remaining portions were stabilized by chaining them back to the existing vertical soil shoring piles. The northern most and eastern portions were lightened (plaster and finishes removed) and a shear wall was added to the northern most section.

This stabilization is not recommended for any long term as weather can further destabilize the site.

### **Mitigation**

In order for construction to proceed, we will remove the walls of the eastern most section and retain them for reuse in their original position in the final structure, exposing the excavator. Stabilize the northern most section and move it to a flat space on the site. Demolish the remaining portions of the floor and remove the excavator. In the final design the walls that were retained (during the stabilization and mitigation) will be placed back to their proposed original locations.

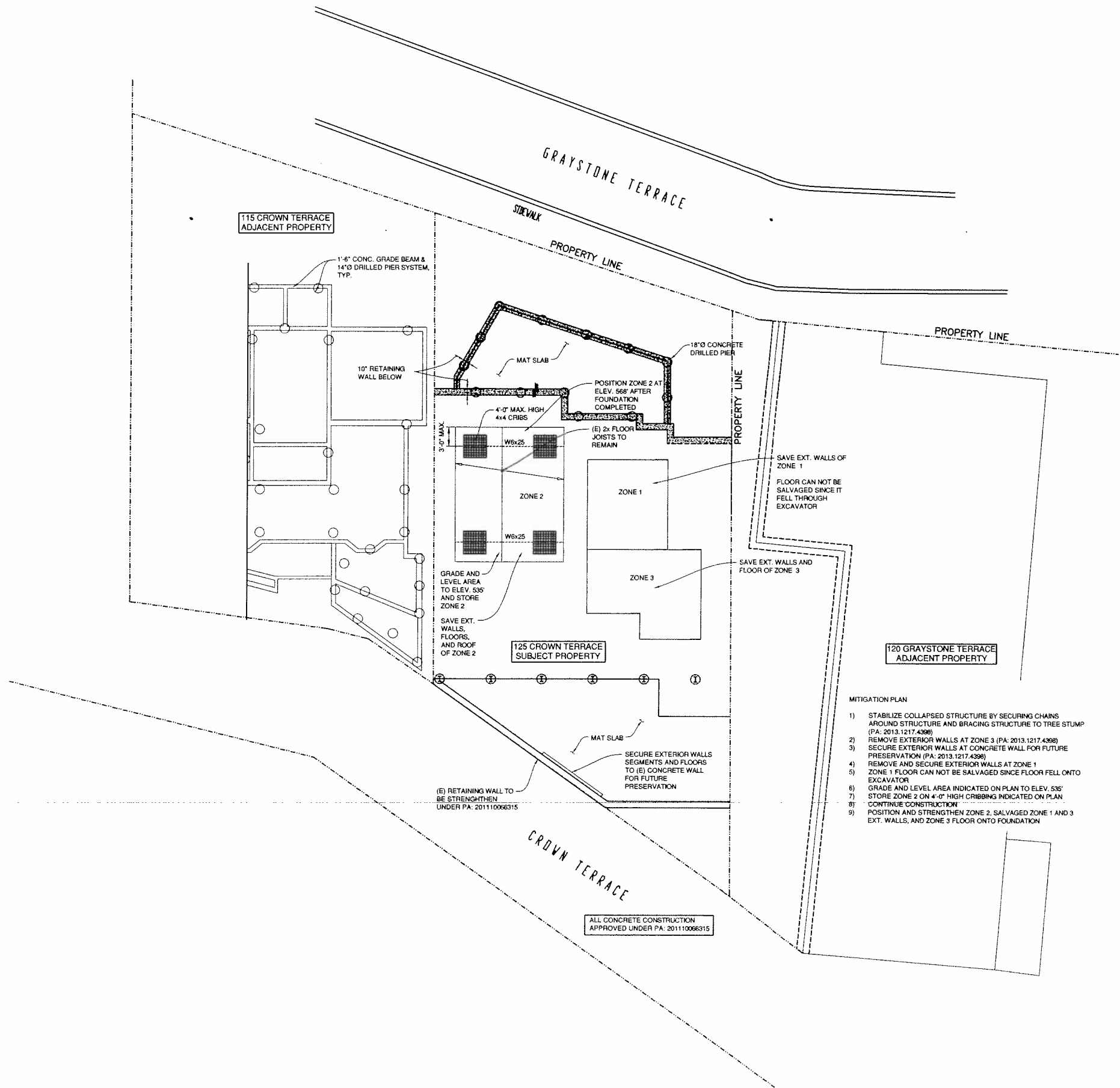
Should you have any questions, please contact me.

Sincerely,

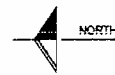
  
Albert Urrutia  
Structural Engineer



Attachments: Collapse Temporary Shoring Plan, Beam Stress Calculations, Photos, Emergency Demolition and Stabilization Plan and Mitigation Plan



SITE PLAN WITH NEW SHORING PLAN @ ELEVATION 556'  
SCALE: 1/8"=1'-0"



REVISIONS	BY

SANTOS & URRUTIA  
STRUCTURAL  
ENGINEERS  
2031 MARINER STREET  
SAN FRANCISCO, CA 94115  
TELEPHONE (415) 842-7722  
FAX (415) 842-7500



RELOCATION & MITIGATION PLAN  
OF COLLAPSED STRUCTURE

MITIGATION  
125 CROWN TERRACE  
SAN FRANCISCO, CALIFORNIA

Date:	12/20/13
Scale:	1/8"=1'-0"
Drawn By:	R.S.
Job No:	9356
Sheet	SK3
Of	3 Sheets

PROJECT NAME: EMERGENCY SHORING/STABILIZATION
ADDRESS: 125 CROWN TERRACE
BLOCK: 2719B LOT: 003
CITY: SAN FRANCISCO, CA
BUILDING USE: SINGLE FAMILY RESIDENTIAL
SCOPE OF WORK: EMERGENCY SHORING AND STABILIZATION OF PARTIALLY COLLAPSED BUILDING.
CLIENT: NAME: MEL MURPHY, ADDRESS: 125 CROWN TERRACE, SF, CA
ARCHITECT & STRUCTURAL ENGINEER: NAME: RODRIGO SANTOS, COMPANY: SANTOS & URRUTIA STRUCTURAL ENGINEERS
GEOTECHNICAL ENGINEER: NAME: HAROLD LEWIS, COMPANY: HAROLD LEWIS & ASSOCIATES GEOTECHNICAL CONSULTANTS
GEOLOGIST: NAME: PATRICK L. DRUMM, COMPANY: EARTH FOCUS GEOLOGICAL SERVICES, INC.

GENERAL STRUCTURAL NOTES
I. GENERAL
A. ALL CONSTRUCTION SHALL CONFORM TO THE CALIFORNIA BUILDING CODE 2010 EDITION w/ AMENDMENTS BY LOCAL JURISDICTIONS.
B. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AT JOB SITE BEFORE COMMENCING WORK AND SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT OR ENGINEER.
C. OMISSIONS OR CONFLICT BETWEEN VARIOUS ELEMENTS OF THE DRAWINGS, NOTES, AND DETAILS SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT AND RESOLVED BEFORE PROCEEDING WITH THE WORK.
D. DO NOT USE SCALED DIMENSIONS; USE WRITTEN DIMENSIONS OR WHERE NO DIMENSION IS PROVIDED, CONSULT THE ARCHITECT FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK.
E. DETAILS SHOWN SHALL BE INCORPORATED INTO THE PROJECT AT ALL APPROPRIATE LOCATIONS WHETHER SPECIFICALLY CALLED OUT OR NOT.
F. FOR WATERPROOFING, FIREPROOFING, ETC. REFER TO DRAWINGS OTHER THAN STRUCTURAL.
G. SEE DRAWINGS OTHER THAN STRUCTURAL FOR: KINDS OF FLOOR FINISH AND THEIR LOCATION, FOR DEPRESSIONS IN FLOOR SLABS, FOR OPENINGS IN WALLS AND FLOORS REQUIRED BY ARCHITECTURAL AND MECHANICAL FEATURES, FOR ROADWAY PAVING, WALKS, RAMPS, STAIRS, CURBS, ETC.
H. HOLES AND OPENINGS THROUGH WALLS AND FLOORS FOR DUCTS, PIPING AND VENTILATION SHALL BE CHECKED BY THE CONTRACTOR, WHO SHALL VERIFY SIZES AND LOCATION OF SUCH HOLES OR OPENINGS WITH THE PLUMBING HEATING, VENTILATING AND ELECTRICAL DRAWINGS AND THESE SUB-CONTRACTORS.
I. NO PIPES AND DUCTS SHALL BE PLACED IN SLABS OR WALLS UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE ARCHITECT.
J. DRAWINGS AND SPECIFICATIONS REPRESENT FINISHED STRUCTURE. SUBCONTRACTOR SHALL BE RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION INCLUDING BUT NOT LIMITED TO SHORING AND TEMPORARY BRACING. THE SUBCONTRACTOR SHALL UNDERTAKE ALL NECESSARY MEASURES TO INSURE SAFETY OF ALL PERSONS AND STRUCTURES AT THE SITE AND ADJACENT TO THE SITE. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT, ENGINEER SHALL NOT RELIEVE THE SUBCONTRACTOR OF SUCH RESPONSIBILITY.
K. NOTE THAT SHEET S1 IS A STANDARD COVER SHEET AND AS SUCH, NOT ALL TYP. DETAILS AND OR NOTES APPLY TO EVERY PROJECT.
II. DESIGN CRITERIA
A. APPLICABLE CODE: CALIFORNIA BUILDING CODE 2010 & SBC 2010.
B. VERTICAL LIVE LOADS: (REDUCIBLE). ROOF: 20 PSF. FLOOR: 40 PSF. HALLWAYS & CORRIDORS: 100 PSF.
III. MATERIALS
A. CONCRETE:
1. REINFORCING STEEL: ASTM A615, GRADE 60, #4 AND SMALLER, GRADE 40.
2. CONCRETE: NORMAL WEIGHT U.O.N. WITH COMPRESSIVE STRENGTH OF THE FOLLOWING AT 28 DAYS:
FOOTINGS, MAT SLAB & DRILLED PIERS 3000 psi
WALLS, COLUMNS 5000 psi
STRUCTURAL SLAB (L.W. P.T. SEE S3.2) 5000 psi
3. MINIMUM CONCRETE COVER FOR REINFORCING STEEL:
a. SURFACE POURED AGAINST GROUND 3"
b. FORMED SURFACES- BELOW GRADE 2"
c. SURFACES EXPOSED TO WEATHER 2"
d. BEAM BARS (INCLUDING STIRRUPS) 1-1/2"
e. ALL OTHER 1"
4. ANCHOR BOLT EPOXY: HILTI HIT-RE 500-SD. (ICC ESR-2322) OR SIMPSON SET-XP (ICC ESR-2508)
5. SCREW ANCHORS: SIMPSON TITEN HD (ICC ESR-2713)
\* USE COMPRESSED AIR TO BLOW THE DUST OUT OF ANCHOR BOLT HOLES.
IV. EXPOSURE TO WEATHER:
A. STEEL:
1. ALL EXPOSED MEMBERS SHALL BE COATED WITH A ZINC RICH PRIMER.
2. BOLTS, NUTS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED.
B. WOOD:
1. ALL EXTERIOR TIMBER AND GLU-LAM BEAMS SHALL BE PRESSURE TREATED (BUT NOT CHROMATED COPPER ARSENATE) OR WOOD OF NATURAL RESISTANCE TO DECAY.
2. ALL EXTERIOR HANGERS AND OTHER SIMPSON TYPE PRODUCTS SHALL BE GALVANIZED.
3. ALL PLYWOOD SHALL BE OF AN EXTERIOR GRADE.
4. METAL CONNECTORS IN CONTACT w/ PRESSURE TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED w/ MIN. ZINC COATING OF 0.185.
5. ALL NAILS & ANCHOR BOLTS IN CONTACT w/ PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED.
V. ABBREVIATIONS
B.N. - BOUNDARY NAILING
DIA. - DIAMETER
E.N. - EDGE NAILING
H.P. - HIGH POINT
L.P. - LOW POINT
LVL - LAMINATED VENEER LUMBER
L.W. - LIGHT WEIGHT
M.L. - MICROLAM
PLWD - PLYWOOD SHEATHING
PSL - PARALLEL STRAND LUMBER
P.T. - PRESSURE TREATED OR POST-TENSIONED
S.A.D. - SEE ARCHITECTURAL DRAWINGS
S.O.G. - SLAB-ON-GRADE
S.S. - STAINLESS STEEL
T.O. - TOP OF
TYP. - TYPICAL
U.O.N. - UNLESS OTHERWISE NOTED
W.W.F. - WELDED WIRE FABRIC
SYMBOLS
(E) WALL BELOW
(E) WALL ABOVE
(N) CONC. WALL ABOVE
WOOD SHEARWALL (BELOW)
WOOD JOIST HANGER (HUS TYPE, U.O.N.)
WOOD POST BELOW
WOOD POST ABOVE (OR ABOVE & BELOW)
HOLDOWN @ WOOD POST
STEEL COLUMN ABOVE
STEEL COLUMN BELOW
MOMENT CONNECTION
CONCRETE TOPPING OVER PLYWOOD
CONCRETE TOPPING OVER CORRUGATED METAL DECK
CONCRETE COLUMN ABOVE
CONCRETE COLUMN BELOW w/ DROP CAP
DRILLED CONCRETE PIER
PRECAST, PRESTRESSED CONCRETE PILE
DIAGONAL ABOVE
DIAGONAL BELOW

City and County of San Francisco
Department of Building Inspection
SPECIAL INSPECTION AND STRUCTURAL OBSERVATION
A COPY OF THIS DOCUMENT SHALL BE KEPT WITH THE APPROVED STRUCTURAL DRAWING SET
JOB ADDRESS: 125 CROWN TERRACE APPLICATION NO.: ADDENDUM NO.
OWNER NAME: MEL MURPHY OWNER PHONE NO.: 415 642-4203
Employment of Special Inspection is the direct responsibility of the OWNER, or the engineer/architect of record acting as the owner's representative. Special Inspector shall be one of those as prescribed in Sec. 1704. Name of special Inspector shall be furnished to DBI District Inspector prior to start of the work for which the Special Inspection is required. Structural observation shall be performed as provided by Section 1710. A preconstruction conference is recommended for owner/builder or designer/builder projects, complex and highrise projects, and for projects utilizing new processes or materials.
In accordance with Sec. 1701;1703;1704 (2010 SBC), Special Inspection and/or testing is required for the following work:
1. Concrete (Placement & sampling)
2. Bolts installed in concrete
3. Special moment-resisting concrete frame
4. Reinforcing steel and prestressing tendons
5. Structural walling
6. Single pane flat glass 5/16" or smaller
7. Steel deck
8. Welded studs
9. Cold formed studs and joists
10. Slab and sitting systems
11. Reinforcing steel
12. Continuous visual inspection and NDT (Section 1704)
13. All other welding (NDT exception: Fillet weld)
14. Reinforcing steel; and 15. NOT required
16. Moment-resisting frames
17. Others
18. High-strength bolting
19. Structural masonry
20. Pultruded steel joists per SBC Sec. 1807C & 1815C
21. Insulating concrete fill
22. Shotcrete
23. Sprayed-on fireproofing
24. Fining, drilled piers and caissons
25. Shotcrete
26. Special grading, excavation and filling (Geotechnical Engineer)
27. Smoke-control system
28. Demolition
29. Exterior facing
30. Retrait of unreinforced masonry buildings (Section 1704.20)
31. Retrait of unreinforced masonry buildings (Section 1704.20)
32. Others: As recommended by professional of record.
33. Others: As recommended by professional of record.
34. Structural observation per Sec. 1710 (2010 SBC) for the following: Foundations, Steel framing, Concrete construction, Masonry construction, Wood framing, Other.
35. Certification is required for: Glass components
Prepared by: Rodrigo Santos Phone: 415 642-7722
Engineer/Architect of Record
Required information:
FAX: 415 642-7722 Email: rsantos@santosurrutia.com
Review by: [Signature] Phone: 415 642-7722
DBI Engineer or Plan Checker
APPROVAL (Based on submitted reports.)
DATE: DBI Engineer or Plan Checker / Special Inspection Services Staff
QUESTIONS ABOUT SPECIAL INSPECTION AND STRUCTURAL OBSERVATION SHOULD BE DIRECTED TO:
Special Inspection Services (415) 558-6132; or, dbi.specialinspection@sfbo.org; or FAX (415) 558-6474
Special Inspection Services
1660 Mission Street - San Francisco CA 94103
Office (415) 558-6132 - FAX (415) 558-6474 - www.sfbdi.org

NOTICE
SPECIAL INSPECTION REQUIREMENTS
Please note that the Special Inspections shown on the approved plans and checked on the Special Inspections form issued with the permit are required for this project. The employment of special inspectors is the direct responsibility of the owner or the engineer/architect of record acting as the owner's representative.
These special inspections are required in addition to the called inspections performed by the Department of Building Inspection. The name of the special inspector shall be furnished to the district building inspector prior to start of work for which special inspection is required.
For questions regarding the details or extent of required inspection or tests, please call the Plan Checker assigned to this project or 415-658-6132. If there are any field problems regarding special inspection, please call your District Building Inspector or 415-558-6570.
Before final building inspection is scheduled, documentation of special inspection compliance must be submitted to and approved by the Special Inspection Services staff. To avoid delays in this process, the project owner should request final compliance reports from the architect or engineer of record and/or special inspection agency soon after the conclusion of work requiring special inspection. The permit will not be finalized without compliance with the special inspection requirements.
STRUCTURAL OBSERVATION REQUIREMENTS
Structural observation shall be provided as required per Section 1710. The building permit will not be finalized without compliance with the structural observation requirements.
Special Inspection Services Contact Information
1. Telephone: (415) 558-6132
2. Fax: (415) 558-6474
3. Email: dbi.specialinspection@sfbo.org
4. In person: 3rd floor at 1660 Mission Street
Note: We are moving towards a 'paperless' mode of operation. All special inspection submittals, including final letters, may be emailed (preferred) or faxed. We will also be shifting to a paperless fax receipt mode.

REVISIONS
BY
SANTOS & URRUTIA INC.
STRUCTURAL ENGINEERS
2451 HARRISON STREET, SF, CA 94110
TELEPHONE: (415) 642-7722
FAX: (415) 642-7722
DEC 17 2013
TOM C. HUI, SE
DIRECTOR
DEPT. OF BUILDING INSPECTION
TITLE SHEET
GENERAL STRUCTURAL NOTES
EMERGENCY SHORING/STABILIZATION
125 CROWN TERRACE
SAN FRANCISCO, CA
Date: 12/17/2013
Scale: N/A
Drawn By: R.S.
Job No: 9356
Sheet S1
Of 2 Sheets



ADDITIONAL SHEETS

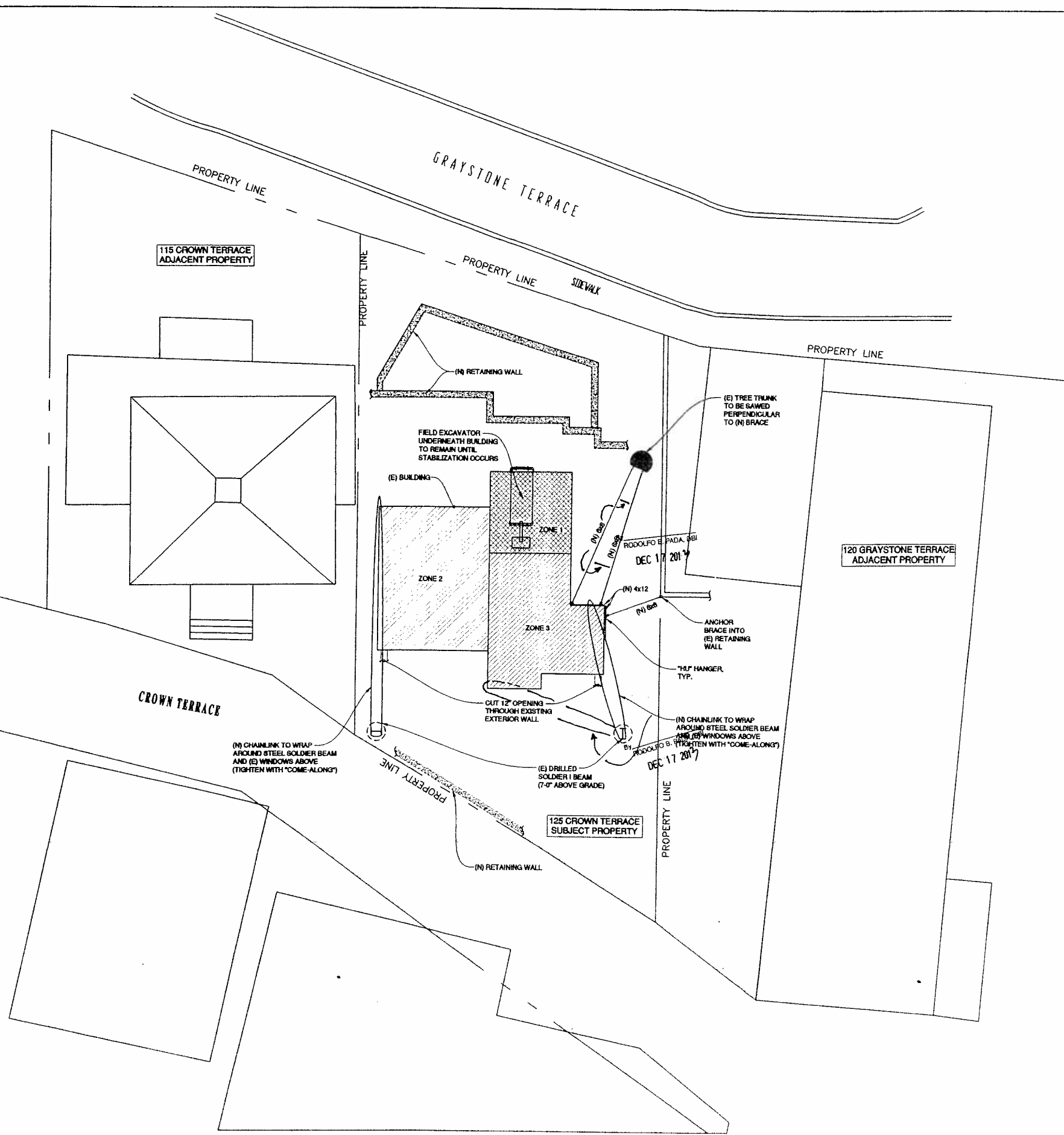
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12/17/2013 11:34 AM Plotted by: osantos

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35x24



- SEQUENCE OF CONSTRUCTION:
1. SHAPE TREE TRUNK PERPENDICULAR TO BRACE
  2. INSTALL (N) 4x12 TO (E) BELLY BEAM WITH SDS SCREWS
  3. INSTALL (N) 6x8 BRACES FROM TREE TRUNK TO (N) 4x12 WITH "HU" HANGERS
  4. INSTALL CHAIN FROM STEEL SOLDIER BEAM TO DEMOLISH ZONE 3 OF STRUCTURE
  5. REVIEW SITE STABILIZATION AND ANALYZE THE STABILITY OF ZONES 1 & 2
  6. IF ZONES 1 & 2 REMAIN UNSTABLE, FURTHER DEMOLITION MAY BE REQUIRED

APPROVED  
Dept. of Building Insp.  
DEC 17 2013  
Tom C. Hui  
TOM C. HUI, S.E.  
DIRECTOR  
DEPT. OF BUILDING INSPECTION

RODOLFO B. PADA, DBI  
DEC 17 2013

SITE PLAN WITH EMERGENCY SHORING/STABILIZATION OF (E) BUILDING  
SCALE: 1/8"=1'-0"



RECEIVED  
DEC 17 2013  
DEPT. OF BUILDING INSPECTION  
ACCEPTED

REVISIONS	BY

SANTOS & URRUTIA  
STRUCTURAL  
ENGINEERS  
245 HARRISON STREET  
SAN FRANCISCO, CA 94110  
TELEPHONE (415) 642-7722  
FAX (415) 642-7590

REGISTERED PROFESSIONAL ENGINEER  
NO. 52884  
EX. 8/20/14  
SANTOS & URRUTIA

EXISTING SITE PLAN  
SEQUENCE OF EMERGENCY SHORING  
& STABILIZATION OF (E) BUILDING

EMERGENCY SHORING/STABILIZATION  
125 CROWN TERRACE  
SAN FRANCISCO, CA

Date:	12/17/2013
Scale:	1/8" = 1'-0"
Drawn By:	R.S.
Job No:	9356
Sheet	S2
Of 2	Sheets

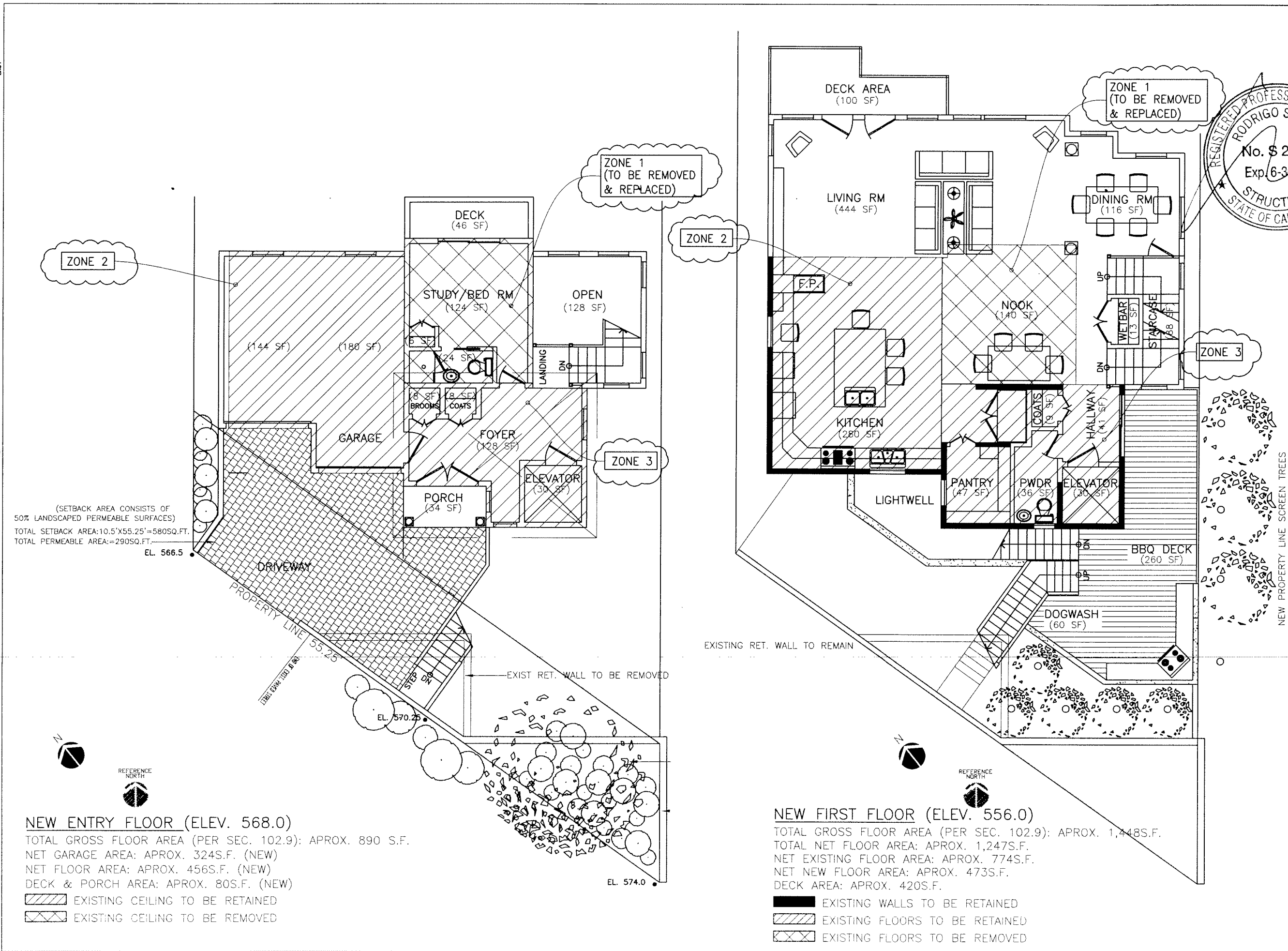
X:\Drawing\8100-8199\8199\Arch\02-03-13 - Current\Arch Plans with ZONES.dwg 2013 4:16 PM: Plotted by: asantos

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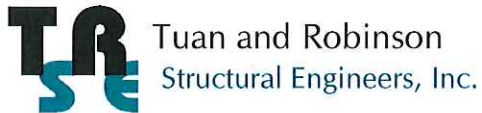
XREF:

36x24 TIME

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REVISIONS	
DATE	
10.01.10	11.01.12
11.15.10	
03.15.11	
03.28.11	
04.25.11	
08.30.11	
03.15.12	
09.24.12	
DESIGN BY: ZONE DESIGN DEVELOPMENT	
DRAKE GARDNER 415.408.3400 (O)	
10 CARULE DR. 415.408.3429 (F)	
NOVATO, CA. 94945 415.377.6694 (C)	
PROJECT: SINGLE FAMILY RESIDENTIAL ALTERATION	
OWNER: CROWN TERR., LLC	
125 CROWN TERRACE 4153 24TH ST., S.F. 94114	
SAN FRANCISCO, CA. 94114	
LOT: 003 BLOCK: 2719B (415) 648-1200	
DATE:	06.24.10
SCALE:	1/4"=1'-0"
DESIGNER:	D.G.
JOB NO.:	
SHEET NO.:	A2



Tuan and Robinson  
Structural Engineers, Inc.

ATTACHMENT F

Eugene Y. Tuan, S.E.  
Alan Robinson, S.E.

Timothy C. Mathison, P.E.  
Steven F. Lew, S.E.

December 31, 2013

Tom C. Hui, SE, CBO  
Director  
Department of Building Inspection  
City and County of San Francisco  
1660 Mission Street, Sixth Floor  
San Francisco, CA 94103

By Electronic Mail

Re: 125 Crown Terrace – SFDBI Structural Advisory Committee  
**Review of Engineering Report in Response to Complaints Nos. 201343501 and 201343861**

TRSE Reference Number: 2013.061.01

Dear Mr. Hui:

As part of the SFDBI Structural Advisory Committee, the following is my review of the Engineering Report for 125 Crown Terrace, dated December 23, 2013, and produced by Santos and Urrutia Structural Engineers, Inc. This report was produced in response to events occurring on December 16, 2013, resulting in Complaints Nos. 201343501 and 201343861.

There are the following comments:

1. The Collapsed Temporary Shoring Plan shows a total of three cribbing towers, a future tower, and two shoring beams. The condition shown on plan differs from the narrative, which indicated three towers along the north side of the building and two towers with a shoring beam along the south side. Please clarify this discrepancy.
2. The Emergency Shoring Plan shows that the existing tree trunk near the southeast corner of the lot will be shaped to accept 6x8 wood members to temporarily brace Building Zone 3. It is suggested that if possible, additional blocks/shims be placed between the tree trunk and the new adjacent retaining wall in the event the tree trunk is not stable. Verify adequacy of 6x8 wood members given their apparent long unbraced length and connections to the existing building. Verify that 6x8 wood members are rigidly attached at floor level to the existing building.
3. Verify if additional lateral loads (wind, seismic, other environmental) need to be considered in design of the temporary building supports during construction.
4. Clarify if additional temporary support is required for the south side of Building Zone 2 (temporary braces, chains, etc.).
5. Verify adequacy of existing soldier beams to resist additional loads from downhill building zones due to proposed chain restraints. Are chains themselves adequate for such loads?
6. The Mitigation Plan sheet indicates that Building Zone 2 will be relocated at Elevation 535' on W6 beams supported on 4'-0" high cribs upon completion of grading and leveling. Verify adequacy of W6 shoring beams and cribs shown and of existing framing to span between W6 beams. Clarify how Zone 2 will be supported during grading and leveling operations.



Tom C. Hui, SE, CBO  
December 31, 2013  
Page 2 of 2

7. The Mitigation Plan sheet also indicates that Building Zone 2 is to be raised to Elevation 568' after completion of the foundation. Confirm if Zones 1 and 3 are to be raised to this level. Will an intermediate shoring plan showing temporary supports at Elevation 568' be required? Submit such a plan if so required with all required documentation.
8. The narrative discusses the addition of a shear wall at the northernmost building segment. This shear wall is not shown on any of the drawings in the report. Clarify intent of shear wall. Is this shear wall designed as a permanent wall for the completed building or is it designed for the temporary condition? Verify adequacy of shear wall design if so required.

Please note that Tuan and Robinson Structural Engineers Inc. makes no warranty, either expressed or implied, as to any findings, designs, recommendations or professional advice except that they were prepared in accordance with generally accepted professional engineering practice.

We trust this is the information you were seeking. Please do not hesitate to call should you have further questions or comments.

Very truly yours

Tuan and Robinson Structural Engineers, Inc.

Steven Lew, SE  
License No. S4180, exp. 9/30/2015

cc: Edward Sweeney, SFDBI  
Hanson Tom, SFDBI





Tuan and Robinson  
Structural Engineers, Inc.

Eugene Y. Tuan, S.E.  
Alan Robinson, S.E.

Timothy C. Mathison, P.E.  
Steven F. Lew, S.E.

January 22, 2014

Tom C. Hui, Director  
Department of Building Inspection  
City and County of San Francisco  
1660 Mission Street, Sixth Floor  
San Francisco, CA 94103

By Electronic Mail

**Subject:: 125 Crown Terrace Shoring Collapse Causation  
SFDBI Structural Advisory Committee**

**Reference:** 1. Shoring Letter Re: Structure Collapse 125 Crown Terrace dated  
12/23/2013 by Santos & Urritia  
2. Temporary Shoring Plan SK1 to SK5 dated 8/30/2013 by Santos & Urritia

Dear Mr. Hui,

The referenced August 8, 2013 Temporary Shoring Plans for the existing structure to remain required the portion of the existing building to remain be shored on steel beams supported by a total of 9 shoring towers placed under the existing building. Required spacing between shoring towers shown on the plans approved by DBI and included in the approved permit drawings was 9 to 12-feet.

The referenced Structure Collapse letter, by Santos & Urritia, the engineer of record for the shoring design, indicates the collapse resulted from buckling failure of an over-stressed steel beam at the south side of the temporary shoring. We note that the Collapsed Shoring Plan attached to the referenced letter indicates only 3 shoring towers were used and shoring beam span between towers was with an approximately 20-feet.

The above-described shoring at the site was not in accordance with the DBI permitted plans. The reduced number of shoring towers, associated increased tower spacing and longer span of the shoring beams is likely the cause of the reported beam failure and subsequent collapse.

Please note that this letter does not address any other comments, findings or directives from SFDBI or from any agency for the City and County of San Francisco concerning 125 Crown Terrace or the Engineering Report.

Very truly yours  
Tuan and Robinson Structural Engineers, Inc

Timothy C. Mathison  
Vice President



cc: Edward Sweeney, SFDBI; Hanson Tom, SFDBI; Frank L Rollo SFDBI SAC Chair





December 31, 2013

Dear Director Hui,

As assigned in accordance with the instructions in the Action Plan dated December 19, 2013, here are our recommendations for 125 Crown Terrace:

- 1) The Engineer of Record (EOR) needs to expand in the report explaining the reasons that the construction sequences are deviated from the approved permit.
- 2) The EOR shall submit to the department all relevant inspection record performed by the Structural Engineer, the Geotechnical Engineer and the Geologist.
- 3) The EOR shall engage and work with the City Planning Department for the purpose of preservation of the existing house; identify existing walls and flooring system to be preserved and used for the new house.  
The EOR shall start this process immediately to avoid prolonging the existing house from exposing to the environment.  
The EOR shall monitor the collapsed house daily and provide addition of supports to secure the house as needed. EOR is required to submit to DBI for permit to perform hazard mitigation work.
- 4) The EOR shall provide responses to the evaluation reports, dated December 31, 2013, prepared by Structural Advisory Committee (SAC) member Steven Lew. The EOR is required to submit to DBI for permit to perform remedial work, as necessary, to meet the requirements of this evaluation report. (Resolved: See Attachment #2)
- 5) The EOR shall submit new permit to revise the permit application #201110066315, especially the structural drawings, to incorporate the reuse of the existing house walls and flooring system.  
In the event the new structural system and the shoring system changed drastically from the approved permit application #201110066315, a SAC shall be required.
- 6) The EOR shall perform items 1, 2, 3 & 4 within one week of the issuance of this report; or, as approved by DBI. In addition, the EOR shall provide a schedule to perform item #5. DBI and City Planning shall review and approve the schedule.



7) Additional Permit Fees to DBI:

125 Crown Terrace: 2011-1006-6315 .... \$24,181.53

115 Crown Terrace: 2013-1008-8806 .... \$2,127.79

From: Ed Sweeney, Deputy Director *ES*  
Hanson Tom, Principal Engineer *HT*  
David Leung, TSD Manager *DL*

Attachments:

- 1) Evaluation Report (by Ed Sweeney, Hanson Tom, David Leung)
- 2) Construction value and permit fee (by Ed Sweeney, Hanson Tom, David Leung)
- 3) SAC evaluation report (by Steven Lew, S.E.)
- 4) EOR reports (by Santos & Urrutia Structural Engineers, Inc.)

# **1) Evaluation Report**

**(by Ed Sweeney, Hanson Tom, David Leung)**



## Evaluation Report:

In response to Santos & Urrutia Structural Engineers' EOR Reports  
Ref: Engineer of Record (EOR) Reports Attached:  
Dated Dec. 23, 2013 & Dec. 30, 2014  
Address: 125 Crown Terrace

Date: Dec. 30, 2013  
To: Tom Hui, Director  
Thru: Ed Sweeney, Deputy Director  
From: Hanson Tom, Principal Engr.  
David Leung, TSD Manager

Dear Director Hui:

### FINDING:

We have reviewed the EOR reports and found the reports are incomplete with many details not totally in agreement with your requests stipulated in your Action Plan dated on Dec. 19, 2013. (Attachment #1)

In your Action Plan to the DBI Commission, you specially required the EOR provide engineering report in explain these items: the "cause of the structure collapse; mitigation plan; and timeline to place the structure back upon its newly-poured foundation".

The EOR report explained the cause of collapse is due to the buckling of one of the cradle beams! The failure of this beam caused the existing house sliding downward to the slope; and the existing house splitting into three portions!

However, the report didn't address the construction sequence which the contractor must follow in accordance with the approved drawings SK-4 and SK-5. In these two drawings, the construction sequences were stipulated by the EOR and were reviewed and approved by the "Structural Advisory Committee (SAC). Nine temporary cribbings have to be in place prior to install the cradle beams to lift the existing house. In addition, these nine cribbings are required to design to resist for environmental loads such as earthquake and wind load. Drawing SK-5 clearly shown the cribbings are to be secured by lateral bracings. However, the field pictures clearly shown there are only few cribbings installed to lift the house upward and there is no lateral bracing in place, which probably is the main cause of the collapse of the temporary supports and the sliding of the house downward of the slope.



There is no indication in the report spelled out the interfacing of the EOR, the Soil Engineer / Geologist and the contractor. The approved drawing clearly stipulated that the shoring contractor is required to work and coordinate with these three parties! Special inspection for shoring is clearly stipulated in the approved drawing S1.0 and the EOR report didn't address this inspection requirement!

Since the existing house collapsed, the EOR has obtained a permit application #201312174398 stabled and also removed some portion of the existing house to mitigate the falling hazards. However, as noted by the EOR, these temporary measures are only useful for the short time. There are no longer term measures stipulated by the EOR to prevent the existing portion of the house from moving further downward!

The EOR stipulated that the collapsed existing house to be removed into three portions and preserved for future restoration back with the new house. The three existing portions identified into three zones, the southern portion as Zone 1, the eastern portion as Zone 2 and the northern portion as Zone 3. Due to the impact of collapse, the southern portion Zone 1 suffered damages beyond preservation and will be demolished; the eastern portion Zone 2 walls to be removed and reused into the final position of the new building; the northern portion Zone 3 to be relocated to a flat area in the site and preserved for future restoration back with the new house. The EOR shown in its sketches how the preserved portions to be used with the new house.

However, upon reviewed the record permit structural drawings S2, S3 and S4 under issued permit application # 201110066315 S1, the framings for the new house are shown with new construction; there is no indication that any existing framings, walls and floor structural elements to be used.

The EOR report didn't include any schedule of submittal of permit application for reconstruction!

## Recommendation:

The staff recommending the following action items for the department:

- 1) The EOR needs to expand in the report explaining the reasons that the construction sequences are deviated from the approved permit.
- 2) The EOR shall submit to the department all relevant inspection record performed by the Structural Engineer, the Geotechnical Engineer and the Geologist.
- 3) The EOR shall engage and work with the City Planning Department for the purpose of preservation of the existing house; identify existing walls and flooring system to be preserved and used for the new house. The EOR shall start this process immediately to avoid prolonging the existing house from exposing to the environment.



The EOR shall monitor the collapsed house daily and provide addition of supports to secure the house as needed. EOR is required to submit to DBI for permit to perform hazard mitigation work.

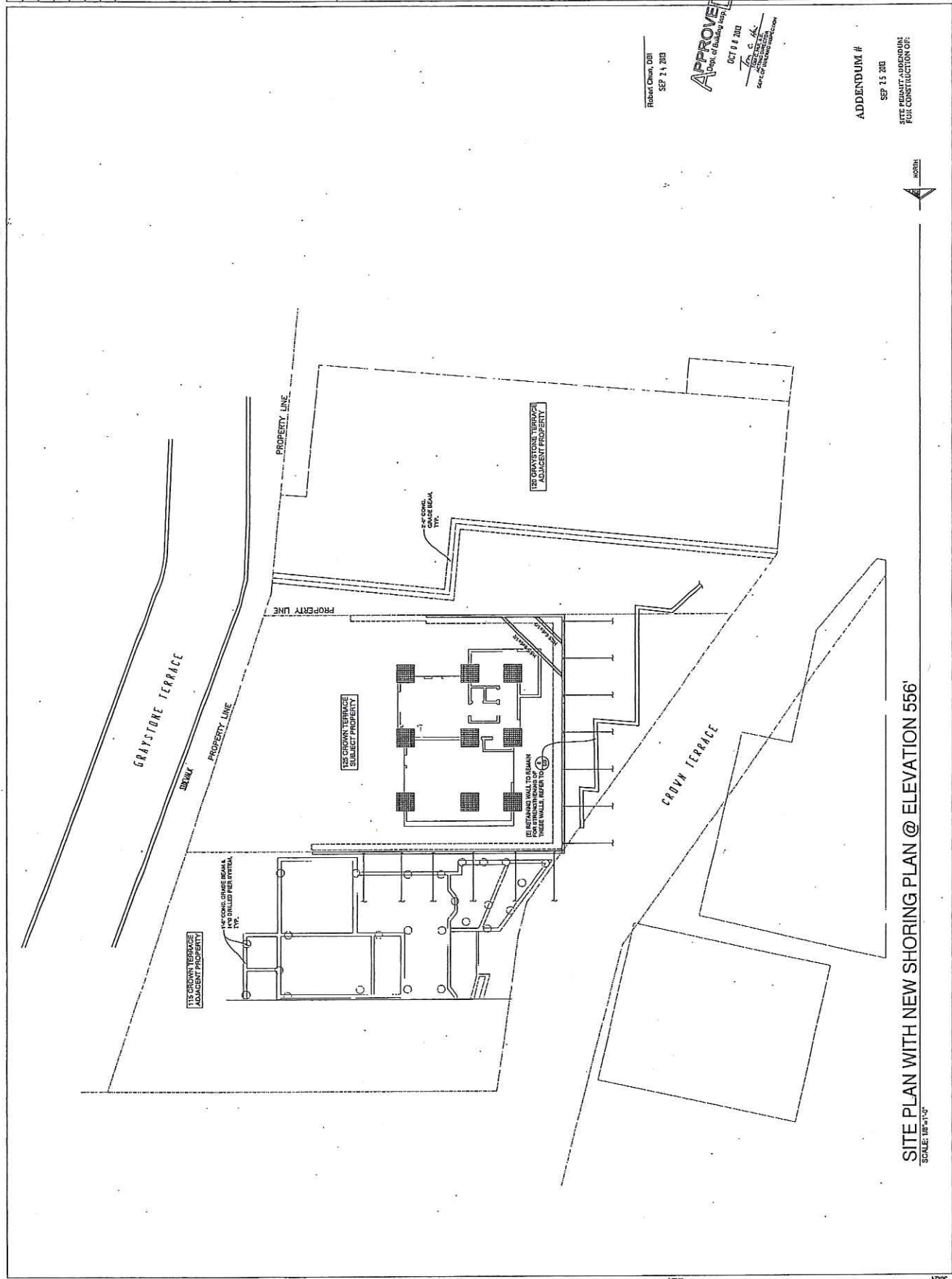
- 4) The EOR shall submit new permit to revise the permit application #201110066315, especially the structural drawings, to incorporate the reuse of the existing house walls and flooring system.

In the event the new structural system and the shoring system changed drastically from the approved permit application #201110066315, a SAC shall be required.

- 5) The EOR shall perform items 1, 2, & 3 within one week of the issuance of this report; or, as approved by DBI. In addition, the EOR shall provide a schedule to perform item #4. DBI and City Planning shall review and approve the schedule.

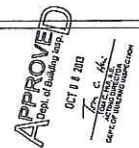


<b>SINGLE FAMILY RESIDENCE</b> <b>125 CROWN TERRACE</b> <b>SAN FRANCISCO, CALIFORNIA</b>		DATE: 08/30/13 SCALE: 1/8"=1'-0" DRAWN BY: R.S. JOB NO: 8180 SHEET: 3 OF 9 SHEETS
<b>TEMPORARY SHORING</b> <b>SUBJECT SITE PLAN W/</b> <b>SHORING PLAN &amp;</b> <b>NEIGHBOR FOUNDATION PLAN</b>		SANTS & URAUTIA STRUCTURAL ENGINEERS 2441 HANCOCK STREET SAN FRANCISCO, CA 94110 TELEPHONE (415) 643-7232 FAX (415) 643-7250



**SITE PLAN WITH NEW SHORING PLAN @ ELEVATION 556'**  
 SCALE: 1/8"=1'-0"

Robert Chen, DBI  
 SEP 21 2013



ADDENDUM #

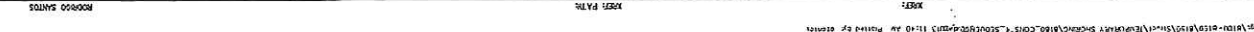
SEP 15 2013  
 SITE PLANNING, ARCHITECTURAL  
 FOR CONSTRUCTION OF

125 GRAYSTONE TERRACE ADJACENT PROPERTY

125 CROWN TERRACE ADJACENT PROPERTY

125 GRAYSTONE TERRACE ADJACENT PROPERTY





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3821







**MEMORANDUM**

**DATE:** December 19, 2013

**TO:** Honorable Members of the Building Inspection Commission

**FROM:** *Tom C. Hui*  
Tom C. Hui, S.E., C.B.O.  
Director

**SUBJECT:** DBI Action Plan to Investigate the 125 Crown Terrace Building  
Collapse of December 16, 2013

As I proposed to the Building Inspection Commission at yesterday's Commission meeting, I am taking the following steps to understand what caused the construction site incident at 125 Crown Terrace, and what the mitigation plan will be to restore the structure to the newly-installed foundation.

- First:** The property owner's engineering firm Santos & Urrutia, will provide to DBI by 3:00 p.m. on Monday, December 23, 2013, its technical report explaining the cause of the structure's collapse and slide down the property slope. This report also will include the engineering firm's proposed mitigation plan, and timeline, to place the structure back upon its newly-poured foundation.
- Second:** The Director instructed the engineer of record to prepare an investigation report to explain the cause of the incident and what is the proposed mitigation and restoration plan in the future by no later than Monday, December 23, 2013. In addition to DBI engineers' review and professional critique of the owner's proposed mitigation and restoration plan, the Director ordered DBI's Deputy Director Edward Sweeney and DBI's Principal Engineer, Hanson Tom, to have a third-party structural engineering peer review to evaluate the technical feasibility of the report. The third-party's structural engineering peer review shall be done by Tim Mathison and Steve Lew of the original peer review group.

With the holidays upon us, we expect this third-party structural peer group evaluation to occur and to provide the DBI Director with its evaluation by no later than December 31, 2013. Once the Director has both this third-party professional report, and the evaluation from DBI's own engineers, the Director will then decide whether or not the owner's mitigation and restoration plan is technically feasible.

**OFFICE OF THE DIRECTOR**  
1660 Mission Street - San Francisco CA 94103  
Office (415) 558-6131 - FAX (415) 558-6225  
Email: Tom.Hui@sfgov.org

*(ATTACHMENT #1)*

# **2) Construction Value and Permit Fees**

**(by Ed Sweeney, Hanson Tom, David Leung)**

## Valuation Fee for 125 Crown Terrace

Permit Application	Original Construction Cost	Final Construction Cost Valuation	Difference in Construction Cost	Permit Fee			Date
				At Filing	At Issuance	Post Issuance	
201110066315	\$ 60,000.00			\$ 2,737.37			10/6/2011
201110066315	\$ 60,000.00	\$ 300,000.00	\$240,000.00		\$ 14,847.34		11/28/2012
201110066315 (Addenda) Receipt #13095920 Receipt #13106615	\$ 300,000.00	\$ 300,000.00	\$0.00			\$2,882.35 \$1,806.54	9/25/2013 10/08/2013
201312174363	\$ 300,000.00	\$ 610,000.00	\$ 310,000.00		\$ 15,270.29		12/17/2013
201110066315 Receipt #1401658 Receipt #1401934	\$ 610,500.00	\$ 1,570,000.00	\$ 959,500.00			\$24,181.53 \$617.22	1/16/2014 1/23/2014
201401216754 NOV 201447604	\$72,198.00	\$ 72,198.00	\$0.00	\$0.00	\$ 6,010.71	\$0.00	1/21/2014

## Valuation Fee for 115 Crown Terrace

Permit Application	Original Construction Cost	Final Construction Cost Valuation	Difference in Construction Cost	Permit Fee			Date
				At Filing	At Issuance	Post Issuance	
201310088806	\$ 25,000.00	\$ 25,000.00			\$974.69		10/9/2013



Code	Description	Account	Date Paid	Fee Amount	Fee Due At	Pay
PLAN REV-P	Plan Review Fee (Postissue)	1961101		8,399.10		<input checked="" type="checkbox"/> ▲
SITE S/C-P	Site surcharge (Postissue)	1961101		1,679.82		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106	10/08/2013	3.12		<input type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106	10/08/2013	32.30		<input type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106		33.60		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106	09/25/2013	56.52		<input type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106		72.00		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106		167.98		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106		196.33		<input checked="" type="checkbox"/>
REC RET-P	Records Retention Fee(POSTISSUE)	1961184	10/08/2013	156.00		<input type="checkbox"/> ▼

Amount paid 4,688.89

Amount to be paid 24,181.53

Total amount due 28,870.42

NEW PAYMENT

RECEIPT

POSTISSUE CHARACTERISTICS

Code	Description	Account	Date Paid	Fee Amount	Fee Due At	Pay
STRG MO-P	Strong Motion Instrumentation Fee	1960645		165.10		<input checked="" type="checkbox"/>
BLDGSTD-P	Bldg Stds Admin Spec Revolv Fund	1961164		51.00		<input checked="" type="checkbox"/>
DCP-P	DCP Plancheck (P)	1960173		9,816.60		<input checked="" type="checkbox"/>
BLDG-P	Bldg Permit Insp Fee (Postissue)	1961115	10/08/2013	1,615.12		<input checked="" type="checkbox"/>
BLDG-P	Bldg Permit Insp Fee (Postissue)	1961115		3,600.00		<input checked="" type="checkbox"/>
PLAN REV-P	Plan Review Fee (Postissue)	1961101	09/25/2013	2,825.83		<input checked="" type="checkbox"/>
PLAN REV-P	Plan Review Fee (Postissue)	1961101		8,399.10		<input checked="" type="checkbox"/>
SITE S/C-P	Site surcharge (Postissue)	1961101		1,679.82		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106	10/08/2013	3.12		<input checked="" type="checkbox"/>
TECH SUR-P	Technology Surcharge (P)	1961106	10/08/2013	32.30		<input checked="" type="checkbox"/>

NEW PAYMENT

RECEIPT

POSTISSUE CHARACTERISTICS

Amount paid 4,688.89

Amount to be paid 24,181.53

Total amount due 28,870.42





# 125 CROWN TERRACE

Pg A of A

## SUMMARY

### (A) 125 CROWN TERRACE

NOTE: 1. ESTIMATION BASED WITH:  
(1) DBE PERMIT RECORD #20110066315  
(2) DBE COST SCHEDULE, MARCH 2009

ITEM	ESTIMATE	PAGE
(1) BLDG AREA	\$ 824,861	6
(2) FOUNDATION	\$ 351,249	10
(3) EXCAVATION AND FILL	\$ 96,578	11
(4) SHOTCRETE/SOIL NAILS	\$ 213,885	12
(5) CONSTRUCTION BERM & RAMP	\$ 29,064	15
(6) TEMPORARY CRIBBING	\$ 43,134	17
TOTAL	\$ 1,558,771	
RECOMMEND TOTAL	\$ 1,550,000	

### (B) 115 CROWN TERRACE

115 CROWN TERRACE (FILE UNDER SEPARATE PERMIT)		PAGE
SHOTCRETE/SOIL NAILS (SOUTH SIDE OF 115 CROWN TERRACE)	\$ 103,000	13



# 125 CROWN TERRACE

PG 1 OF 17

## FLOOR AREA

DRGS: A1-A9, AB1-AB4

(F) NEW ENTRY FLOOR (ELEV. 568.0) (NOTE: ALL NEW FRAMING)  
SEE S-DWG'S.

(A) GARAGE AREA

$$(17' \times 20.5') - (4.33' \times 9') = \underline{310 \text{ FT}^2} \leftarrow$$

(B) HABITABLE AREA

$$B = [(12.5' + 11')(13.5' + 13.75')] - \left[ \overset{\text{COPED SPACE}}{(4' \times 9.75')} + \overset{\text{Porch Elev.}}{(5.75' \times 13.5')} + \overset{\text{Elev.}}{(8' \times 14')} - (5.5' \times 5.5') \right]$$

$$628.6 - 327$$

$$= \underline{302 \text{ FT}^2} \leftarrow$$

(C) DRIVEWAY + PORCH AREA

$$C = [(17' + 8')(12') - (4.25')(16')] + \overset{\text{MEASURED}}{[(1/2)(25')(9')]} + [(4')(12.5')]$$

$$= \underline{394 \text{ FT}^2} \leftarrow$$

(D) Elevator stop

(ONE STOP!)  $\leftarrow$

(E) ONE 3/4 BATHROOM  $\leftarrow$

(F) ONE FLIGHT OF STAIR  $\leftarrow$   
TO LOWER DECK (CONC.)